

**Concrete
Operations**

Lesson 7



Standard and Special Provisions to be familiar with:

- 00512 – Drilled Shafts
- 00530 – Steel Reinforcement for Concrete
- 00540 – Structural Concrete
- 02001 – Concrete
- 00290 – Environmental Protection

Documents that you will need:

- Drilled Shaft Inspector's Checklist
- Drilled Shaft Installation Plan
- Drilled Shaft Inspector's Report
- Drilled Shaft Concrete Volumes
- Drilled Shaft Concrete Volume Logs

Concrete Operations Learning Objectives

- Concrete basics
 - mix design
 - concrete placement
 - concrete sampling and testing
- Concrete Placement Log
- Concrete Volume Log
- Inspector's roles, and responsibilities



Inspector's Roles and Responsibilities

Concrete Operations

<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	31. Prior to concrete placement, has the slurry (both manufactured and natural) been tested in accordance with Section 00512.43(g)??
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	32. If required, was the casing removed in accordance with Section 00512.47(e)?
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	33. Does the Contractor's tremie meet the requirements of Section 00512.47(a)?
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	34. Was the discharge end of the tremie maintained in the concrete mass with proper concrete head above it at all times (00512.47(c))?
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	35. For shafts with non-contact splices, have the cold joints been properly cleaned and roughened in accordance with Section 00512.47(a)?
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	36. For shafts without non-contact splices, did the Contractor overflow the shaft until good concrete flowed out of the top of the excavation (00512.47(a))?
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	37. Have the Concrete Placement and Concrete Volume logs been completed?
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	38. Were the concrete acceptance tests performed as required?
<input type="checkbox"/> Yes	<input type="checkbox"/> No	<input type="checkbox"/> NA	39. Were the Crosshole Sonic Log (CSL) tubes filled with water and capped in accordance to Section 00512.46?



Change to checklist image.

Concrete Basics

What do you think are the desired characteristics of drilled shaft concrete?

- Workability
- Self-consolidating
- Nominal aggregate size
- Temperature
- Strength



Concrete Basics

What are the concrete properties, tolerances, and limits?

Standard Provisions 02001 Concrete

- Strength
- Air entrainment
- Slump
- Temperature



OSSC 2015

00512.12 Concrete Mix Design - Design the drilled shaft concrete for minimum segregation. Use the Engineer's reviewed and approved mix design.

- Add water to the concrete mix at the Project Site only if allowed by the approved mix design. Accurately measure water added at the site by water meters, buckets or other approved devices. Limit the addition of water at the Project Site to 1 gallon per cubic yard.
- Provide concrete having the appropriate initial slump according to Table 02001-3. Use chemical admixtures from the QPL to control and maintain slump and to facilitate temporary casing extraction.
- Design the concrete mix to maintain at least 4 inches of slump after placement and throughout the entire duration of the pour including during temporary casing extraction.

Concrete Basics

2001 concrete properties, tolerances and limits (continued)

Strength required for Drilled Shafts

(a) Strength – Provide concrete meeting the required Classes shown in the Contract Documents. The class of concrete designates the minimum required compressive strength, f'_c at 28 days, and the nominal maximum size of aggregate to be used in the concrete (for example, Class 3300 - $\frac{3}{4}$: f'_c is 3,300 psi with a nominal maximum size aggregate of $\frac{3}{4}$ -inch).

Table 02001-1

Concrete Strength and Water/Cementitious Material (w/cm) Ratio		
Type of Concrete	Strength (psi)	Maximum w/cm Ratio
Structural	3300	0.50
	3300 (Seal)	0.45
	4000	0.48
	4000 (Deck)	0.40
	HPC4000	0.40
	5000 and above	0.40
	HPC5000 and above	0.40
Drilled Shaft	4000	0.48
Paving	4000	0.44



Concrete Basics

Required concrete strength?

Special Provisions

00512.10(b) Concrete – Replace this subsection, except for the subsection number and title, with the following:

Use Class 4000 drilled shaft concrete according to Section 02001, except as modified in this Section. Water may be added to the concrete mix at the Project Site only if allowed by the approved mix design.



Concrete Basics

2001 concrete properties, tolerances and limits (continued)

Air Entrainment required for Drilled Shafts

(b) Air Entrainment – Provide all concrete, except PPCM with cast-in-place decks, seal concrete, and drilled shaft concrete with entrained air in the amounts shown in Table 02001-2. Field measured entrained air content shall be within ± 1.5 percent of target air entrainment values.



Concrete Basics

2001 concrete properties, tolerances and limits (continued)

Slump required for Drilled Shafts

Table 02001-3

Concrete Slump	
Condition	Slump
Concrete without WRA	4" max.
Concrete with WRA	5" max.
Concrete with HRWRA	5 1/2" ± 2 1/2"
Precast Prestressed Concrete with HRWRA	10" max.
Seal Concrete	8" ± 2"
Drilled Shaft Concrete	8 1/2" ± 1 1/2"



Concrete Basics

2001 concrete properties, tolerances and limits (continued)

Temperature required for Drilled Shafts

(d) Temperature – Provide concrete, at time of placement, at a temperature between a minimum of 50° F and a maximum of 90° F, except the maximum bridge deck concrete temperature shall be 80° F.



Concrete Basics

What are the concrete mix design criteria?

Standard Specifications

00512.12 Concrete Mix Design – Design the drilled shaft concrete for minimum segregation. Use the Engineer’s reviewed and approved mix design.

- Add water to the concrete mix at the Project Site only if allowed by the approved mix design. Accurately measure water added at the site by water meters, bucket, or other approved method. Limit the addition of water at the Project Site to the concrete batching plant yard.
- Provide concrete having a slump according to Table 02001-3. Use chemical admixtures from the QPL to control and maintain slump and to facilitate temporary casing extraction.
- Design the concrete mix to maintain at least 4 inches of slump after placement and throughout the entire duration of the pour including during temporary casing extraction.

Concrete Basics

What are the concrete mix design criteria?

Special Provisions

00512.10(b) Concrete – Replace this subsection, except for the subsection number and title, with the following:

Use Class 4000 drilled shaft concrete according to Section 02001, except as modified in this Section. Water may be added to the concrete mix at the Project Site only if allowed by the approved mix design.

00512.12 Concrete Mix Design – Delete this subsection.



**What does a
Concrete Mix Design look like?**



MIX DESIGN
Rose Mount Sand & Gravel Co.
6112 W. McLaughlin Blvd
P.O. Box 82349-0249
Portland, OR 97282

Rose Mount Sand & Gravel Co.
88001.5th AGRGREGATE, 8" SLUMP, NO AIR, 4 HOUR MIX (2)
Strength Compressive: 4000 psi
05/27/2016

Project: ODOT NUMBER SOURCE: BYPASS		Contractor: PACIFIC FOUNDATION	
Compressive Strength:	4000 psi at 28 days	Source of Concrete:	Rose Mount Sand & Gravel Co.
Aggregate Size:	3/4" - 5.0 mm	Consolidation Type:	RODDED METHOD
Air:	0.5 to 5.0 %	Placement:	PUMP OR CAST
Maximum Temp:	65-85	Min Weight:	100% wet
Slump:	1" to 10" 00	Design Date:	05/20/16
M			

Constituents	Quantity	Density	Volume
Type I/II Cement (Cement)	550 lb	1.500	2.87
ASTM C-595 Gravel 100 (kg) (coarse)	140 lb	2.800	0.81
Water	300 lb	1.000	4.87
ASTM #40 Sand (Aggregates)	1300 lb	2.700	8.82
ASTM C-33 Sand (Aggregates)	1800 lb	2.700	9.81
ASTM C-404 Fly Ash (ASTM)	90.00 lbs (0.0000)	1.120	0.81
Denon Stabilizer (ASTM)	70.00 lbs (0.0000)	1.000	0.87
M		2.5 %	0.84
Total		4011	27.00

Notes:
SP. AGG. SP. GRAVITY (SSD) 2.703 DRY RODDED UNIT WEIGHT 107.30 FINE AGG. SP. GRAVITY (SSD) 2.700 FINE AGG.

Reported by: Wayne Flues Approved by: WAYNE FLUES ODOT CCT# 4418
Date: 05/20/16 Date: 05/10/16

OSSC 2015

02001.35 Required Submittals for Mix Designs - Submit the following information for each concrete mix design:

(a) Supplier's Unique Mix Design Identification Number

(b) Mix Design Constituent Proportions:

- Weight per cubic yard (pounds per cubic yard) of cementitious material, modifiers, fine and coarse aggregates (SSD), and mix water.
- Absolute volumes of cementitious material, modifiers, fine aggregates and coarse aggregates (SSD), and mix water.
- Dosage rates for chemical admixtures.

(c) Aggregates - Identify the aggregate source by the ODOT source number. Report current values of the following:

- Bulk specific gravities (SSD)
- Fine aggregate absorptions
- Coarse aggregate absorptions
- Dry-rodded density of coarse aggregates
- Fineness modulus of sand used in the mix design calculations

(d) Cementitious Material - For each cementitious material used, identify the following:

- Manufacturer
- Brand name
- Type
- Relevant Specification
- Source or location plant

(e) Modifiers - For each modifier used, identify the following:

- Manufacturer
- Brand name
- Source
- Relevant specification
- Class

(f) Admixtures - For each admixture used, identify the following:

- Manufacturer
- Brand name
- Design dosage rate

(g) Water - Identify the source of water to be used.

(h) Plastic Concrete Tests - Report the temperature, slump, density, air content, yield, and w/cm ratio of the trial batch or the average of these values for the cylinder sets presented for evaluation of a current mix design.

(i) Compressive Strength Test Results - Report the individual test results and the ASTV of cylinders from the trial batch or the average for the cylinder sets presented for evaluation of a current mix design.

(j) Strength Analysis - Provide an analysis, showing all calculations, demonstrating that the mix design meets the requirements of 02001.33.

(k) Quality Control Personnel - Provide the name and certification number of the CCT who prepared the mix design, the QCT who performed the plastic concrete tests and cast the test cylinders, the laboratory where the cylinders were tested, and the CSTT who tested the cylinders.

Test Results

ODOT
 Ross Island Sand & Gravel Co.
 4315 SE McLoughlin Blvd P.O. Box 82249-8249
 Portland, OR 97226
 8301 - 3/8" AGGREGATE, 8" SLUMP, NO AIR, 6 HOUR MIX [0]

Cast Date	Sample ID	Slump	W/Ap	Temp	Unit Weight	W/C Ratio	Yield	Day 1 PSI	Day 7 PSI	Day 28 PSI	Day 56 PSI	Day 90 PSI	Avg 1 - 28 Day
1/15/2015	KEHR	5.50	2.10	58.00	151.20	0.48	0.50	4520	1010	1010			1010
2/25/2015	KEHR	5.75	1.90	58.00	150.50	0.43	5.99	8750	9103	9103			9103
2/24/2015	KEHR	8.00	1.70	68.00	151.00	0.41	8.86	6360	10460	10460			10460
2/26/2015	Extreme Ex	5.25	2.20	57.00	151.20	0.42	8.93	6820	11183	11183			11183
2/25/2015	KEHR	6.25	2.10	58.00	148.40	0.42	7.57	5980	10527	10527			10527
2/26/2015	Extreme Ex	5.00	1.50	60.00	151.00	0.41	7.43	5190	9310	9310			9310
3/30/2015	KEHR	7.75	1.40	54.00	150.70	0.43	8.85	4760	9213	9213			9213
3/6/2015	Extreme Ex	7.25	1.50	60.00	151.40	0.41	8.88	6210	10590	10590			10590
3/16/2015	KEHR	7.00	1.20	67.00	150.80	0.41	8.90	7910	8203	8203			8203
3/17/2015	KEHR	7.00	1.30	60.00	150.40	0.41	8.98	8770	9640	9640			9640
3/18/2015	KEHR	7.75	1.20	69.00	150.20	0.42	9.91	5410	8473	8473			8473
4/26/2015	KEHR	6.00	1.80	69.00	151.10	0.41	10.91	5820	8853	8853			8853
4/19/2015	KEHR	6.25	1.90	73.00	154.20	0.41	9.73	6080	8797	8797			8797
8/17/2015	Extreme Ex	7.00	1.70	77.00	147.40	0.40	7.10	5790	7227	7227			7227
8/25/2015	Paclite Fo	0.00	0.00	82.00	0.00	0.00	0.00	4870	6753	6753			6753
8/31/2015	Paclite Fo	0.00	0.00	79.00	0.00	0.00	0.00	4890	7717	7717			7717
9/15/2015	Paclite Fo	0.00	0.00	82.00	0.00	0.00	0.00	4460	6777	6777			6777
9/20/2015	Paclite Fo	0.00	0.00	82.00	0.00	0.00	0.00	5040	8833	8833			8833
9/20/2015	Paclite Fo	0.00	0.00	81.00	0.00	0.00	0.00	5170	6980	6980			6980
9/8/2015	Paclite Fo	5.00	1.40	77.00	0.00	0.00	0.00	5040	6720	6720			6720
9/11/2015	Paclite Fo	6.50	1.40	81.00	0.00	0.00	0.00	4650	6740	6740			6740
9/19/2015	Paclite Fo	0.00	0.00	77.00	0.00	0.00	0.00	4630	8197	8197			8197
9/19/2015	Paclite Fo	8.00	1.20	78.00	0.00	0.00	0.00	4200	7117	7117			7117
9/18/2015	Paclite Fo	8.50	1.80	79.00	0.00	0.00	0.00	4060	6090	6090			6090
9/17/2015	Paclite Fo	8.00	2.70	80.00	0.00	0.00	0.00	4640	7933	7933			7933
9/17/2015	Paclite Fo	8.00	1.90	78.00	0.00	0.00	0.00	5290	7263	7263			7263
9/18/2015	Paclite Fo	8.50	1.40	81.00	0.00	0.00	0.00	4290	6877	6877			6877
9/21/2015	Paclite Fo	7.00	1.80	77.00	0.00	0.00	0.00	3370	6527	6527			6527
9/21/2015	Paclite Fo	7.50	1.80	78.00	0.00	0.00	0.00	5170	6857	6857			6857
9/24/2015	Paclite Fo	7.50	1.80	81.00	0.00	0.00	0.00	4880	6967	6967			6967
9/25/2015	Paclite Fo	7.00	1.80	79.00	0.00	0.00	0.00	5090	6790	6790			6790
9/25/2015	Paclite Fo	6.50	1.80	78.00	0.00	0.00	0.00	5260	6790	6790			6790
9/28/2015	Paclite Fo	8.00	2.10	80.00	0.00	0.00	0.00	3370	6690	6690			6690
10/1/2015	Paclite Fo	9.00	1.80	79.00	0.00	0.00	0.00	3890	5233	5233			5233
10/2/2015	Paclite Fo	7.00	1.70	67.00	0.00	0.00	0.00	5720	8877	8877			8877
10/8/2015	Paclite Fo	8.00	0.80	79.00	0.00	0.00	0.00	4460	6870	6870			6870
10/8/2015	Paclite Fo	8.00	1.80	78.00	0.00	0.00	0.00	4290	6900	6900			6900
10/8/2015	Paclite Fo	8.00	1.80	78.00	0.00	0.00	0.00	4560	6887	6887			6887
10/12/2015	Paclite Fo	7.50	1.50	78.00	0.00	0.00	0.00	3750	6760	6760			6760
10/13/2015	Paclite Fo	9.00	3.20	78.00	0.00	0.00	0.00	4450	6810	6810			6810
4/18/2016	Paclite Fo	0.00	0.00	8.00	0.00	0.00	0.00	5050	7323	7323			7323
4/18/2016	Paclite Fo	8.50	1.70	75.00	0.00	0.00	0.00	5700	7930	7930			7930
4/18/2016	Paclite Fo	8.00	1.40	77.00	0.00	0.00	0.00	5800	7240	7240			7240
4/20/2016	Paclite Fo	8.50	1.20	75.00	0.00	0.00	0.00	5700	7887	7887			7887
4/21/2016	Paclite Fo	8.00	1.30	73.00	0.00	0.00	0.00	5880	7983	7983			7983
4/22/2016	Paclite Fo	7.00	1.80	70.00	0.00	0.00	0.00	5340	7933	7933			7933
4/25/2016	Paclite Fo	8.50	1.30	68.00	0.00	0.00	0.00	5400	8250	8250			8250
4/26/2016	Paclite Fo	8.50	1.80	69.00	0.00	0.00	0.00	5	6610	6610			6610
4/28/2016	Paclite Fo	7.00	1.50	69.00	0.00	0.00	0.00	5530	8037	8037			8037
		7.81	1.50	76.00	153.71	0.42	8.82	4831	7416	7416			Average
		1.00	0.38	8.00	1.54	0.01	1.30	1013	1421	1421			STD
		5.50	2.80	42.00	6.80	0.06	4.92	5420	8906	8906			Range
		4.00	0.40	54.00	147.40	0.49	3.99	2960	4257	4257			Low
		9.50	3.20	96.00	154.20	0.46	10.91	8880	11183	11183			High

Number of observations: 49

Workability

<u>TIME</u>	<u>SLUMP</u>
HOUR 1	9.5
HOUR 2	8.25
HOUR 3	7
HOUR 3.5	7
HOUR 4	7
HOUR 4.5	6.25
HOUR 5	5.75
HOUR 5.5	5
HOUR 6	4.5



OREGON DEPARTMENT OF TRANSPORTATION
 MATERIALS LABORATORY
 800 AIRPORT RD. SE. SALEM, OR 97301-4792

Page 1 of 1
 (503)586-3000
 FAX(503)586-3094

Contract No.: REG 1 QA EA No.: QA1000 000 Lab No.: 15-003432
 Project: REGION 1 COMMERCIAL SOURCE
 Highway: County: Data Sheet No.: P44525 045
 Contractor: City: PA No.:
 Project Manager: BARR WASHINGTON Org Unit: 7840 Bid Item No.:
 Submitted By: GREGORY MURPHY Org Unit: 81 Sample No.:
 Material Source: WA-020-2 AVERY FIT (WA-20-002-4) Qty Represented: 12 MONTHS
 Sampled At: BARGE 15 Sampled By: Witnessed By: ATHA DEB
 DATE Sampled: 15/11/19 Received: 15/11/19 Tested: 15/11/19 Date Reported: 15/11/19
 Class/Type: COMPLIANCE Use: COARSE FCC AGGR

Order G: GRAVEL		AGGREGATE LABORATORY REPORT - CPCCAG		Size: 1/2-84
Test	Field	Lab	T 84 F. Grav.	T 85 C. Grav.
T 176 S.S.			Bulk: 2.705	Bulk: 2.705
T 89 L.L.			S.S.D.: 2.763	S.S.D.: 2.763
T 90 P.T.			Appar.: 2.864	Appar.: 2.864
T 335 Tl. Frac.			Absorp.: 2.00 %	Absorp.: 2.00 %
TM 226 Dast/Clay			T 104 Soundness	TM 208 Degrade
TM 227 Cleaness			C.A. 88 F.A:	
TM 229 Elong pos			1.5-3/4: 0.0 %	
308 Inclin/Oa A/C			3/4-3/8: 1.4 %	
Total A/C			3/8- #4: 6.7 %	Crse Ht: 0.8 in
T 329 Moisture			#4- #8:	P20: 15.9 %
T 27/11			#8-#16:	Fine Ht:
Sieve	Passing	Passing	#16-#30:	P20:
2.5			#30-#50:	T 21 Impurity
2			T 96 Abrasion	Plate #:
1.5			15.5 %	
1			Type C	T 112 Friables
3/4	100 %		T 335 Fracture	1.5:
1/2	94 %		1.0:	1.5-3/4:
3/8	69 %		3/4:	3/4-3/8:
1/4	24 %		1/2:	3/8- #4:
# 4	10 %		3/8:	#4-#16:
# 8	2 %		T 113 Lightweight	TM 225 Nonwaste
# 10			Coarse: 0.1 %	Lab:
# 16			Fine:	Field:
# 30			AAASTO T 288/289	AAASTO T 267
# 40			Resist: 0	Organic:
# 60			AAASTO T 291	AAASTO T 290
#100			Chloride:	Sulfate:
#200			0.6 %	
T 327 Micro Deval ==> Grading:			Loss: %	

2 # t27 = \$ 47.00	NON = Not Sufficient Material	TOTAL CHARGES: \$ 0.00
2 # t11 = 23.00	REMARKS:	
1 # t85 = 45.00	Material represented by sample DOES comply with specifications.	
1 # t56 = 97.00		
3 # t104 = 29.00		
1 # t113 = 34.00		
1 # t208 = 74.00		

KEVIN BROPHY - LABORATORY SERVICES MANAGER

REPORT SHALL NOT BE REPRODUCED, EXCEPT IN FULL, WITHOUT WRITTEN APPROVAL OF THIS LABORATORY.

C: FIELDS, GREGORY MURPHY - BARGE ISLAND BRIDGE & GRAVEL, REGION 1 QA; J: JOHNSON - CONCRETE QUALITY; J: CIBULAK - ADMINISTRATIVE

OREGON DEPARTMENT OF TRANSPORTATION Page 1 of 1

MATERIALS LABORATORY (503)586-3000

800 AIRPORT RD. SE. SALMON, OR 97301-4792 FAX (503)586-3096

Contract No.: REG 1 QA EA No.: QA1000 000 Lab No.: 15-003433

Project: REGION 1 COMMERCIAL SOURCE - Data Sheet No.: P44525 046

Highway: FA No.:

Contractor: Project Manager: BARD WOODRINGTON Org Unit: 7840 Bid Item No.:

Submitted by: GREGORY HOPKIN Org Unit: 81 Sample No.:

Material Source: MA-020-2 AVERY PIT (MA-20-002-4) Qty Represented: 12 MONTHS

Sampled At: BARGE 15 Sampled By: Licensed By: ATMA MSD

DATE-Sampled: 15/10/10 Received: 15/11/10 Tested: 15/11/10 Date Reported: 15/11/10

Class/Type: COMPLIANCE Use: FINE PCC AGGR

AGGREGATE LABORATORY REPORT - PFCAG

Test	Field	Lab	Size	SAND
T 176 S.S.			T 84 P. Grav.	T 85 C. Grav.
T 89 L.L.			Bulk: 2.672	Bulk:
T 90 P.T.			S.S.D.: 2.730	S.S.D.:
Fineness Modulus		2.87 PM	Appar.: 2.838	Appar.:
TM 226 Durz/Clay			Absorp.: 2.20 %	Absorp.:
TM 227 Cleanness			- T 104 Soundness	TM 208 Degrade
TM 229 Elong pos			C.A. F A1 8%	
308 Incln/da A/C			1.5-3/4:	
Total A/C			3/4-3/8:	
Retention			3/8- #4: 0.9 %	Crse Ht:
T 329 Moisture			#4- #8: 3.1 %	#20:
T 27/11	Passing	Passing	#8-#16: 2.3 %	Fine Ht:
2.5			#16-#30: 1.0 %	#20:
2			#30-#50: 0.8 %	
1.5			T 96 Abrasion	T 21 Impurity
1				Plate #: 1
3/4			T 335 Fracture	T 112 Friables
1/2			3/4:	SC'd Log:
3/8	100 %		1/2:	1.5-3/4:
1/4	99 %		3/8:	3/4-3/8:
# 4	95 %		1/4:	3/8- #4:
# 8	78 %		#10:	#4-#16:
# 10			T 113 Lightweight	TM 225 Woodchaste
# 16			Course:	Lab:
# 30	50 %		Fine: 0.5 %	Field:
# 40			AASTO T 288/289	AASTO T 267
# 60	22 %		Resist: 0	Organic:
# 100	8 %		AASTO T 291	AASTO T 290
# 200	2.3 %		Chloride:	Sulfate:
			T 327 Micro Deval	Grading:
				Loss: %

1 # L27 = \$ 47.00 NEM = Not Sufficient Material TOTAL CHARGES: \$ 0.00

1 # L11 = 23.00 **REMARKS:**

1 # L21 = 16.00 Material represented by sample DOES comply with specifications.

1 # L84 = 57.00

5 # L104 = 29.00

1 # L113 = 34.00

KEVIN HOPKIN - LABORATORY SERVICES MANAGER

REPORT SHALL NOT BE REPRODUCED, EXCEPT IN FULL, WITHOUT WRITTEN APPROVAL OF THIS LABORATORY.

BY FIELD: GREGORY HOPKIN - BARGE DELAND SAND & GRAVEL (REGION 1 QA) / J. JOHNSON - CONCRETE QUALITY / J. CHELSEA - ASSISTANT



ASH GROVE
 ASH GROVE CEMENT COMPANY
 WESTERN REGION
 3000 SHEETAL, GIBSON ROAD
 P.O. BOX 387
 DUNES, OREGON 97005
 (503) 871-2411

Duke Plant
MS Test Report

MS Analytical: 10-13 Contact Type: TEST Date: 06-08-2016
 MS No.: 53-147 Production Period: Site 1, Box Log 21, 2016

STANDARD REQUIREMENTS
 ASTM C150

CHEMICAL				PHYSICAL			
Item	Spec. Limit	Test Result	Item	Spec. Limit	Test Result		
SO ₃ (%)	A	28.7	Air Content of Mortar (vol. %)				
Al ₂ O ₃ (%)	6.0 max.	2.6	C185	12 max.	6.0		
Fe ₂ O ₃ (%)	6.0 max.	2.2	Fineness (m ² /kg)				
CaO (%)	A	64.2	C204 (Air permeability)	240 min.	393		
MgO (%)	6.0 max.	2.0	Autoclave Expansion (%)	0.80 max.	0.081		
SiO ₂ (%)	D	2.4	C151				
Loss On Ignition (%)	3.0 max.	2.02	Compressive Strength (psi) (4 days)	Min.			
Na ₂ O (%)	A	0.19	C109	1 Day	A	1180 (21.6)	
K ₂ O (%)	A	0.48		3 Days		1740 (22.0)	2480 (27.8)
TiO ₂ (%)	A	0.28		7 Days		2760 (31.3)	4020 (33.3)
P ₂ O ₅ (%)	A	0.15					
Mn ₂ O ₃ (%)	A	0.07	Time of Setting (minutes)				
Soluble Residue (%)	0.75 max.	0.56	C191 (Flow)				
CO ₂ (%)	A	1.56					
Latent Heat %	5.0 max.	3.80					
CaCO ₃ in Latent Heat	70 max.	99.25					
CSB + 4 TSCIA	350 max.	79					
Potential Compounds (%)	C						
CS	A	60					
CS	A	14					
CSA	8.0 max.	4					
CAAP	A	10					
CAMP (NCL)	A	18					

OPTIONAL REQUIREMENTS
 ASTM C150 (cont)

CHEMICAL			PHYSICAL		
Item	Spec. Limit	Test Result	Item	Spec. Limit	Test Result
Equivalent Alkaline (%)	0.80 max.	0.51	Water Retention (%)	30 min.	101
Chloride (%)	B	0.020	First of Hydration (m ² /kg)	C181	
				7 days	B
			Compressive Strength (psi)		
				28 Days	4020 (28.3)
			Sulfate Resistance (%)	C62	0.042
			Water Expansion (%)	C108	0.020
			Expansion (in 4 days) (mm)	B	0.20

A = not applicable
 B = Test results represent actual tested value and is permitted for inferential purposes only.
 C = Adjusted per A. 1.6.
 D = C108 expansion in water does not exceed 0.02 at 14 days.
 F = Test results for this production period not yet available.

We verify that the above described content, at the time of shipment, meets the chemical and physical requirements of the ASTM C150-10 or ASTM C150-12 Type 1-IT specification also will meet CSA A3000-12 Type G11, M6 and H6.

Signature: *Christoph M. DeWitt*

Title: Plant Chemist





Location Portland Slag Product Dura Slag Date 7-Apr-16
 Certification No. Slag 1-16

STANDARD REQUIREMENTS
ASTM C989 & AASHTO M302

CHEMICAL			PHYSICAL		
Item	Spec. Limit	Test Result	Item	Spec. Limit	Test Result
Slag			Slag		
Sulfide sulfur as S (%)	2.5 max	1.16	% Retained 325 mesh	20 max	3.3
			Blaine fineness (m ² /kg)	A	404
			Air Content of Slag Mortar (%)	12 max	4.5
			Specific Gravity	A	2.89
Reference Cement			Reference Cement		
Total Alkalies (%)	0.60 min 0.90 max	0.77	% Passing 325 mesh	A	92.0
Potential (%)			Blaine fineness (m ² /kg)	A	361
C/S	A	60	Compressive strength MPa (PSI)	min:	
C/S	A	12	7 Days	A	30.8 (4460)
C/S	A	7	28 Days	35 (5000)	40.2 (5830)
C/S	A	8			
C/S	A	22	60-60 Blend of Slag and Reference Cement		
C/S	A	93	Compressive strength MPa (PSI)	min:	
			7 Days	A	30.5 (4430)
			28 Days	A	55.1 (7270)
			Slag Activity Index		
			Grade 120	min:	
			Average of Last 5 Samples 7 Days	A	99
			Any Individual Sample 7 Days	A	96
			Average of Last 5 Samples 28 Days		115
			Any Individual Sample 28 Days		110

*Not applicable

The Ground Granulated Blast-Furnace Slag meets the chemical and physical requirements of the ASTM C989/C99M-14 and AASHTO M 302-13 specifications for Grade 120.

Signature:  Title: Chief Chemist
 Edward C. Rafacz

3801 E Marginal Way S, Seattle, WA 98134; Ph: (206) 623-5596; Fax: (206) 623-5355

Client Sample Results									
Client: Ross Island Sand & Gravel					TestAmerica Job ID: 580-56437-1				
Project/Site: Water									
Client Sample ID: River Water					Lab Sample ID: 580-56437-1				
Date Collected: 01/11/16 11:15					Matrix: Water				
Date Received: 01/11/16 12:09									
General Chemistry									
Analyte	Result	Qualifier	RL	MDL	Unit	D	Prepared	Analyzed	DI Fac
Chloride	4.6		0.55		mg/L			01/16/16 17:48	1
Sulfate	4.2		1.2		mg/L			01/16/16 17:48	1
Analyte	Result	Qualifier	RL	RL	Unit	D	Prepared	Analyzed	DI Fac
Alkalinity	29		5.0		mg/L			01/14/16 09:49	1
Bicarbonate Alkalinity as CaCO3	29		5.0		mg/L			01/14/16 09:49	1
Carbonate Alkalinity as CaCO3	ND		5.0		mg/L			01/14/16 09:49	1
Hydroxide Alkalinity as CaCO3	ND		5.0		mg/L			01/14/16 09:49	1
Total Dissolved Solids	48		10		mg/L			01/12/16 18:08	1
Total Suspended Solids	5.2		2.0		mg/L			01/12/16 18:12	1
pH	7.56	HF	0.0100		SU			01/12/16 20:10	1

Concrete Mix Design

Knife River Materials (Rabg Division)R4000FD5

Specified Strength = 4000

Required Strength = Specified Strength + 1.34 X Standard Deviation

Required Strength = 4591.2

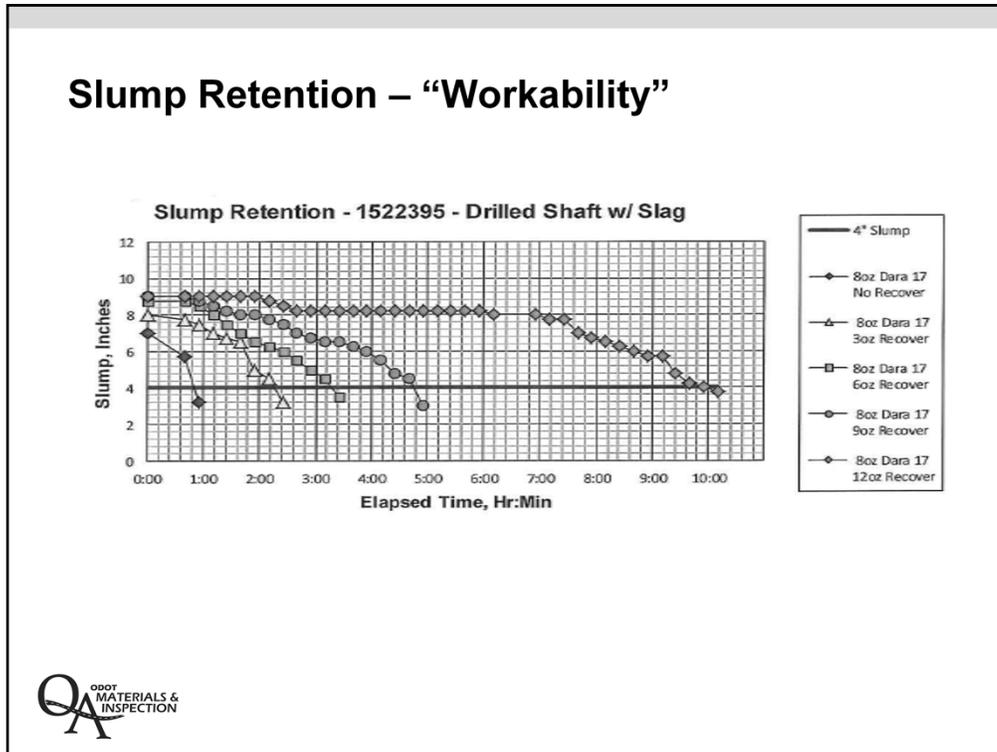
Average of 28 day breaks = 6375.3 which exceeds Required Strength

STDEV SUM A1:A25 AVG Strength AIR AVG W/C AVG Unit Wt. AVG
 441.2 55630 6375.3 3.3 0.44 148

Plastic Concrete Test Results

Specified Targets							
	7 day PSI	28 day PSI	Slump Inch	Air (%)	Temp (F)	W/C Ratio	Unit Wt. (lb/ft ³)
		4000 (±10%)	NA			0.45	143.7
Test Data							
Date	7 day PSI	28 day PSI	Slump Inch	Air (%)	Temp (F)	W/C Ratio	Unit Wt. (lb/ft ³)
9/22/2009	3280	7320	9	4.5	87	0.45	166.9
4/13/2010		3780.0	9.5	3.3	61.2	0.46	147
4/13/2010		3870.0	9.5	3.2	61.3	0.46	147.5
4/13/2010		6120.0	9	3.8	64	0.45	147.1
4/13/2010		6120.0	9	3.8	64	0.45	147.1
4/14/2010		6120.0	9	3.8	64	0.45	147.1
4/14/2010		6120.0	9	3.8	64	0.45	147.1
4/15/2010		6840.0	7.5	3.7	65.4	0.43	147.1
4/15/2010		6650.0	8	3.8	64.5	0.43	147.1
4/16/2010		6420.0	8	3.8	66.7	0.44	147.5
7/12/2010	4140	6330.0	8.75		83	0.45	148.7
7/12/2010	4050	6330.0	8.75		81	0.46	148.6
7/12/2010		7100.0	8.75		83	0.43	148.7
7/13/2010	3810	3980.0	10		74	0.44	147.5
7/13/2010	4380	6450.0	9		76	0.42	147.5





4" Minimum slump has been changed to 6" in OSSC 2015

OSSC 2015

SP00512.12 Concrete Mix Design - Delete this subsection.

SP02001.20(c) Slump - Add the following paragraph to the end of this subsection:

For drilled shaft concrete, maintain a minimum slump of 6 inches throughout the drilled shaft placement, including temporary casing extraction.

SP02001.35(h) Plastic Concrete Tests - Add the following paragraph and bullets to the end of this subsection:

For drilled shaft concrete, report the following additional information:

- The total time estimate from initial batching through drilled shaft placement, including haul time, placing concrete, and temporary casing extraction.
- Initial slump test results and subsequent results at 15-minute intervals, verifying a minimum slump of **6 inches** is maintained for the total time estimated for drilled shaft placement, including temporary casing extraction. Report data in a table or graph format.

Concrete Mix Design

- Aggregate
 - Coarse aggregate
 - Fine aggregate
- Water
- Cement



Concrete Mix Design

MIX DESIGN QUANTITIES:

Material	Product/Source	English Units			Metric Units	
		Spec Grav	Weight	Volume (ft ³)	Mass	Volume (m ³)
Cement	Lehigh	3.150	530 lb	2.70	314 kg	0.100
Fly Ash	Lehigh Type F Centralia	2.560	132 lb	0.83	78 kg	0.030
Silica Fume	Master Builders	2.200	0 lb	0.00	0 kg	0.000
Water (Total)	City	1.000	285 lb	4.57	169 kg	0.169
		2.620 *	0 lb*	0.00	0 kg*	0.000
3/4"-#4 Round	Round Prairie Source #10-029-3	2.743 *	0 lb*	0.00	0 kg*	0.000
3/8"-#4 Round	Round Prairie Source #10-029-3	2.728 *	1430 lb*	8.40	848 kg*	0.311
Fine Aggregate	Round Prairie Source #10-029-3	2.658 *	1549 lb*	9.34	922 kg*	0.347
Admixtures	Master Builders	1.000	13 lb	0.21	8 kg	0.008
Total Mix Weight:			3939 lb		2339 kg	
Air (Entrap/Entrain)		3.5 %		0.95		0.035
Total Mix Volume:				27.00		1.000



Concrete Mix Design

ADMIXTURES:

Product	Product Name/Type	Dosage Rate	Dosage (English)	Dosage (Metric)
Air Entrainment	MB-AE 90	0.00 oz/cwt**	0.0 oz/cy**	0 mL/m3**
Water Reducer	PolyHeed 997	12.0 oz/cwt**	79.0 oz/cy**	3069 mL/m3**
Mid-Range WR	PolyHeed 997	0.0 oz/cwt**	0.0 oz/cy**	0 mL/m3**
Superplasticizer	PS 1466	6.0 oz/cwt**	33.0 oz/cy**	1278 mL/m3**
Retarder		0.0 oz/cwt**	0.0 oz/cy**	0 mL/m3**
	Delvo	15.0 oz/cwt**	99.0 oz/cy**	3833 mL/m3**
Fibers		0.0 lb/cy**	0.0 lb/cy**	0.0 kg/m3**



Concrete Mix Design Review

**OREGON DEPARTMENT OF TRANSPORTATION
MATERIALS LABORATORY
900 AIRPORT ROAD SE
SALEM, OR 97301-4790**

503.908.3099
Fax: 503.908.3096

Contract No.: **C14831** EA: **CON03764** F.A. No. **STP-C-057009** Lab No. **15-CMD005**

Project Name: **Nehalem River (Miami-Foley Rd) Br (Lommen)** Amendment 1 Date:
 Highway: **Miami-Foley Road** Amendment 2 Date:
 County: **Tillamook** Amendment 3 Date:

Contractor: **Fairline Bridge Inc** Material Source: **Nehalem Bay Sand & Gravel**
 Project PM: **Ian Machan** Mix Type: **General Structural**
 ODOT PMS: **William Jablonski** Specified Strength: **6500**
 Submitted by: **Cyndi Twite** Maximum 30"
 Date Received: **10/29/2015** Exposure:
 Date Reported: **10/30/2015** Proposed Use: **Civilian Shaft**

STRUCTURAL CONCRETE MIX DESIGN REVIEW

Mix Design by: **Kevin Wolf** CCT # **41433** Contractor Mix Design No.: **400005A**

Concrete Manufacturer	Concrete Source	Concrete Type	Density (pcf)
Carbond	Reading	FA	160

Modifier	Manufacturer	Modifier Source	Modifier Type	Value
1	Large	Concrete	Fly Ash Class C/F	300
2				
3				

Coarse Agg Source	SSD	Abs	FMV	DREGW	Coarse Agg Size
1	05-004-1	2.632	2.2%	309.9	1/2" - 0
2					
3					

Fine Agg Source	SSD	Abs	FMV	Fine Agg Size	
1	05-004-1	2.617	2.5%	275	#1 - 0
2					
3					

Water Source	Value
Well	300

Admixture Brand	Admixture	Drawer Analysis
SP-1 MasterSet Dens	MSA and Retarding	MS

Slump (in)	Value
8.5	

Air Content (%)	Value
1	

Density (pcf)	Value
148.9	

W/C Ratio	Value
0.44	

Average Trial Batch Compressive Strength 5590 psi @ 7 days
 *Need 28 day breaks

Slump Retention Data @ 98 cu yd:
 10.5" @ 0 Hrs
 07.5" @ 2 Hrs
 05.5" @ 4 Hrs
 04.5" @ 6 Hrs

Based on the information submitted for review, this mix design Does Comply with specifications. This report does not supersede, delete or amend the Contract Documents or relieve the Contractor of the responsibility to provide concrete within specifications.

Scott D. Nelson, P.E. 10/30/2015
 Structural Services Engineer Date
 C-Project Manager, Fairline Bridge Inc Austin Johnson Scott Nelson Eric Burns Cyndi Twite

Concrete Mix Design

Can you “transfer” an approved concrete mix design from one job to another?



The Drilled Shaft mix design needs to follow the concrete mix design submittal requirements of 02001. Consequently - there really is no "transfer" meaning here's a mix design (Lab Report) from this other project -> please assign it to this current project. There are a number of concerns with the "transfer" approach. We've had instances where a mix design approved on a project is then approved on other projects sometimes without any direct contact or involvement from the Concrete Producer. This could be a problem if the Concrete Producer wants or needs to change an additive or update the physical properties of materials. So there's a little more to it than just passing a recipe.

A recent example - a previously approved version of a Drilled Shaft mix used GGBFS instead of fly ash due to a shortage of fly ash when that particular mix design was reviewed. The Concrete Producer had since gone back to fly ash but I had a request to "transfer" the Drilled Shaft mix design with GGBFS. You see the problem?

So the contractor needs to send in a complete submittal for each Contract.

As you now or are learning ~ Drilled Shaft concrete has two critical submittals. The Drilled Shaft mix design which is evaluated according to 00512.12 and 02001 and the Drilled Shaft Installation Plan evaluated according to 00512.40 Submittals.

I'm sure that some people have assumed that an approved mix design = an approved Drilled Shaft Installation Plan. NOT TRUE! I do not review nor approve the Drilled Shaft Installation Plan. The mix design (being a separate submittal) is but one aspect of the Drilled Shaft Installation Plan. Therefore - the mix design submittal should typically precede the Drilled Shaft Installation Plan.

One of the things that I look for concerns the slump retention (workability) requirement of the specifications. The concrete producer may or may not identify the anticipated duration of the drilled shaft placement. However, there will usually be an additive used to hold the slump as required. The exact details about duration of the placement - are to be included in and assured by the Drilled Shaft Installation Plan which has its own 21-day review period.

Call me if you still have any questions.

Austin Johnson
503.986.5461

Concrete Mix Design

How much water is the contractor allowed to add to the concrete at the job site?

OSSC 2015

~~**00512.12 Concrete Mix Design** - Design the drilled shaft concrete for minimum segregation. Use The Engineer's reviewed and approved mix design.~~

- ~~• Add water to the concrete mix at the Project Site only if allowed by the approved mix design. Accurately measure water added at the site by water meters, buckets or other approved devices. Limit the addition of water at the Project Site to 1 gallon per cubic yard.~~



Contractor may want to add water to the concrete at the job site for various reasons:

- increase workability
- increase slump
- change the temperature

In adding water to the concrete the mix design is effectively changed. By adding water this changes important concrete parameters such as:

- water/cement ratio
- strength
- slump
- density

OSSC 2015

~~**00512.12 Concrete Mix Design** - Design the drilled shaft concrete for minimum segregation. Use The Engineer's reviewed and approved mix design.~~

~~• Add water to the concrete mix at the Project Site only if allowed by the approved mix design. Accurately measure water added at the site by water meters, buckets or other approved devices. Limit the addition of water at the Project Site to 1 gallon per cubic yard.~~

SP00512.12 Concrete Mix Design - Delete this subsection.

02001 and **SP2001** does not allow for the addition of water unless it is approved in the mix design!

Concrete Placement

Let the documentation begin!

Drilled Shaft Inspector's Checklist
This checklist to follow when constructing a drilled shaft. The answer to each of these questions should be "Yes" or "NA" unless given otherwise. Any answer of "NA" should be explained in the Notes/Comments.
SUBJECT RESPONSIBILITIES.

DRILLED SHAFT INSPECTION REPORT

DRILLED SHAFT EXCAVATION LOG

DRILLED SHAFT CONCRETE VOLUMES

DRILLED SHAFT CONCRETE PLACEMENT LOG

QA
ODOT MATERIALS & INSPECTION

Concrete Placement

Completed documentation

- What is the placement procedure? What do you need to know?
 - Drilled Shaft Installation Plan
 - Equipment
 - On-site testing
 - Who?



Concrete Placement

Concrete Placement in a Dry Shaft

What is dry?

00512.47 (b) Dry Shaft Concrete Placement Concrete may be placed by free-fall if all of the following conditions are met:

- No more than 3 inches of water is present in the bottom of the excavation at the beginning of the pour
- Groundwater seepage into the excavation is at a rate of no more than 12 inches per hour
- Shaft diameter is greater than or equal to 3 feet



OSSC 2015

00512.47 Concrete – furnish and place concrete according to the following:

(b) Dry Shaft Concrete Placement - Concrete may be placed by free-fall if all of the following conditions are met:

- no more than 3 inches of water is present in the bottom of the excavation at the beginning of the pour
- groundwater seepage into the excavation is at a rate of no more than 12 inches per hour
- shaft diameter is greater than or equal to 3 feet

Under free-fall placement, deposit concrete through the center of the reinforcement cage by a method which prevents segregation of aggregates and splashing of concrete on the reinforcement cage. Place concrete so that the free-fall is vertical down the center of the shaft without hitting the sides, the steel reinforcing bars or steel cage bracing.

Concrete Placement

Equipment Dry Shaft

- Drop chute
(8" minimum diameter)
- Pumped Tremie
(4" minimum diameter)



What are the safety issues?

Tie-off,
hard hat.

Note Beaver Ball – contaminated concrete

Concrete Placement

Dry Shaft Concrete Placement:

What is the allowable free fall distance?

00512.47(b) Under free-fall placement, deposit concrete through the center of the reinforcement cage by a method which prevents segregation of aggregates and splashing of concrete on the reinforcement cage. Place concrete so that the free-fall is vertical down the center of the shaft without hitting the sides, the steel reinforcing bars or steel cage bracing.



Concrete Placement

Wet Shaft Concrete Placement Equipment

- Tremie 4" minimum diameter
 - Gravity/free fall is not allowed in a wet shaft
 - Pressurized
 - Plug/pig
- Equipment
 - Pump



Placement of concrete by freefall into a wet shaft is not allowed.

OSSC 2015

00512.47 Concrete - Furnish and place concrete according to the following:

(c) Wet Shaft Concrete Placement - If the drilled shaft excavation does not meet the requirements for dry concrete placement, stabilize water inflow and place the concrete under water or slurry with a tremie pipe or pump hose according to 00540.48(e). Place concrete continuously from the bottom of the shaft to the top-of-shaft elevation shown. Use a plug in the tremie pipe or pump hose to force water or slurry ahead of the advancing flow of fresh concrete. Dispose of all displaced water, slurry, or waste concrete according to 00290.20. When groundwater, the drilling water or slurry in the shaft excavation is to be removed by pumping during concrete placement, have a standby pump available. Place concrete in a continuous operation so that the concrete always flows upward within the shaft. Withdraw the delivery hose or pipe slowly as the elevation of the fresh concrete rises in the shaft. Keep the discharge **end of the pipe or hose at least 5 feet below the surface of the concrete** after the concrete has reached a depth of 5 feet. Maintain sufficient concrete inside the hose or pipe to prevent drilling fluid from entering. During concrete placement, provide and maintain markings on the tremie pipe or pump hose, or a sounding device or other appropriate method to determine the relative elevations of the fresh concrete surface and the bottom end of the pipe or hose. Raise the bottom end of the pipe or hose only when the pipe or hose has a sufficient head of fresh concrete to prevent the formation of a void at the bottom.

Concrete Placement

What are important factors in using a tremie?

- Initially plugged
- Maintained a minimum of 5-foot head at all times
- Diameter of the tremie



Concrete Placement

- “Place concrete continuously until concrete at the top of the shaft is free of water, soil, and debris, and uncontaminated concrete extends to the plan of top-of-shaft elevation.”
- Is the casing being removed too fast?



There must be enough concrete to “overflow” the shaft to push out the water, soil, etc. until there is uncontaminated concrete.

This picture shows an example where “overflow” is not happening.

OSSC 2015

00512.47 Concrete - Furnish and place concrete according to the following:

Place concrete continuously until concrete at the top of the shaft is free of water, soil, and debris, and uncontaminated concrete extends to the plan top-of-shaft elevation. Dispose of all contaminated concrete expelled from the top of the shaft in an approved manner. Remove waste concrete from the site. If a delay in concrete placement occurs because of a delay in concrete delivery or other factors, reduce the placement rate to maintain a flow of fresh concrete into the shaft excavation

Concrete Placement

- Minimum of 10 feet of concrete above the bottom of casing before the start of removal

AND

- Maintain at least 5 feet above the bottom of casing at all times



OSSC 2015

(e) Casing Removal - Remove all temporary casing during or after completion of concrete placement. Do not start temporary casing removal until the level of fresh concrete within the casing has reached a depth of at least 10 feet or the level necessary to adequately counteract the external hydrostatic pressure head. As the temporary casing is withdrawn, maintain a minimum 5 feet head of concrete above the bottom of the casing. A slight downward movement of the casing while exerting downward pressure, or hammering or vibrating the casing will be allowed to facilitate extraction.

Extract the casing so that concrete is cast directly against the surrounding in-situ material. Check the elevation of the top of the reinforcing cage before and after temporary casing extraction for conformance with the construction tolerance criteria of 00512.42. Casing that cannot be extracted during, or immediately after, the concrete placement operation may be cause for rejection of the shaft.

Remove the tops of permanent casing to the top of the drilled shaft or the finished groundline, whichever is lower, unless otherwise shown or directed. Remove the tops of permanent casing for shafts constructed in a permanent body of water to the low water elevation, unless otherwise shown or directed.

Concrete Placement

How much time does the contractor have to batch, transport and place the concrete?

00512.47(a) Concrete Placement – Unless otherwise approved by mix design, allow a maximum of 60 minutes between concrete placements and use no concrete older than 90 minutes from batch time.



Concrete Placement – On-site Testing

- What tests are performed for drilled shaft concrete?
 - Slump
 - Temperature



On first test, at ODOT have Region come out and do Verification testing through the Region Quality Assurance Coordinator.

Concrete Placement – On-site Testing

(b) Quality Control Technician (QCT):

Duties:

- Attend pre-placement meetings for bridge deck pours and paving.
- Be at the concrete placement site when concrete placement is in progress.
- Have a copy of the mix design on-site and available during concrete placement.
- Obtain and check each batch ticket upon arrival of the concrete at the jobsite for the correct mix design.
- Sample the concrete and test for ambient air temperature, plastic concrete temperature, slump, air content, density, w/cm ratio and yield at the frequencies required by and according to the tests listed in the MFTP, after concrete mixture proportions are adjusted in the field, and at such times as requested by the Engineer.



Concrete Placement – On-site Testing (continued)

(b) Quality Control Technician (QCT):

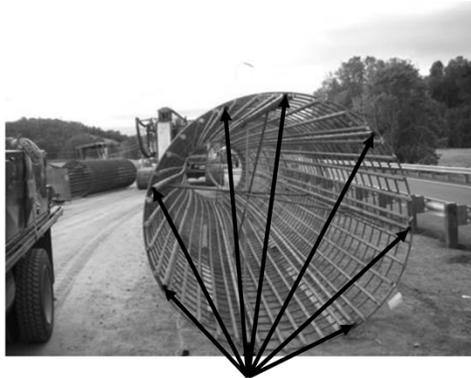
Duties:

- Notify the Contractor and the Engineer immediately when the concrete is not in compliance with the Specifications.
- Be in direct contact with the CCT by telephone, radio or other means to convey information.
- Notify the CCT of loads rejected and the reason for rejection.
- Notify the CCT immediately whenever the density of the plastic concrete varies from the mix design target by more than ± 3 pounds per cubic foot.
- Notify the CCT immediately whenever the w/cm ratio varies from the mix design target by more than ± 0.03 .



Concrete Placement

- Check that CSL tubes were installed as per the plans and filled with water and capped as soon as possible after the concrete pour is completed.
- Before testing, check to see that the water level in the tubes has not dropped.



CSL Tubes



Filling the CSL tubes with water allows the CSL tubes to remain at the same temperature as the concrete so that they do not debond from the concrete.

Debonding from the concrete will result in erroneous CSL test results.

Crosshole sonic log testing requires access tubes cast into the shaft concrete and attached inside the steel cage. The access tubes for CSL testing are always steel to provide the best possible bond between the tube and the concrete. These tubes need to be cast into the concrete at the time of construction. The tubes must be filled with water to dissipate heat from concrete curing process. Without water in the tubes they may debond from the concrete rendering them useless for testing.

It is very important that the tubes are installed on the proper spacing and maintain a straight alignment the full length of the shafts. The results of the test are affected by the distance between each pair of tubes.

Different tests require different types of tubes (e.g., PVC for gamma-gamma versus schedule 40 steel for CSL). Some tests require that the tubes are filled with water, and some don't.

OSSC 2015

00512.46 Crosshole Sonic Log Test Access Tubes - Furnish and install access tubes for CSL

testing as shown. Attach CSL access tubes securely to the interior of the reinforcement cage as near to parallel as possible in each drilled shaft and in the pattern shown. Extend the access tubes from the bottom of the reinforcement cage to at least 24 inches above the top of the shaft. Joints required to achieve full-length access tubes shall be watertight. Do not damage the access tubes during reinforcement cage installation and concrete placement. **Fill the tubes with potable water**, according to 02020.10(b), as soon as possible, but no more than one hour after concrete placement and reinstall the top watertight caps. Check water level and top off as needed.

Replace all access tubes that the test probe cannot pass through to the full depth of the shaft at no additional cost to the Agency. Replace all damaged access tubes with 1.5 inch to 2.0 inch diameter holes cored through the concrete for the entire length of the shaft. Unless otherwise directed, locate replacement core holes approximately 6 inches inside the reinforcement. Do not damage the shaft reinforcement during coring operations.

Fill the access tubes with grout only after all CSL testing has been completed and the shaft has been accepted.

Concrete Placement

PRIOR to Arrival of Concrete




DRILLED SHAFT CONCRETE VOLUMES

PROJECT	BRIDGE NO.	CONTRACT NO.	
RENT	STATION	SHAFT NO.	SHAFT DIAMETER
DRILLED SHAFT CONTRACTOR	INSPECTED BY	CERT. NO.	DATE

CONCRETING CURVE

Prior to pouring concrete, a plot should be made showing the theoretical concrete surface (by depth or elev.) vs. concrete volume placed. During concrete placement the actual concrete surface vs. the actual concrete volume placed is then plotted.

Shaft Top

DEPTH/ELEVATION (Feet)

Shaft Bottom

CONCRETE VOLUME PLACED (cubic yards)

VOLUME CALCULATIONS		Notes/Comments:
Volume Delivered	TVD _____ cy	_____
Volume in Lines	VL _____ cy	_____
Wastage	WV _____ cy	_____
Volume Placed	VP _____ cy	_____
(VP = TVD - VL - WV)		
Theoretical Volume	VT _____ cy	_____
(VT = (PI x Shaft Length) / 27)		
Overpour (VP-VT)	OP _____ cy	_____

OSSC2015

00512.40 Submittals - Provide the following submittals to the Agency for review and approval:

(c) Drilled Shaft Inspection Reports - Provide the Engineer with a completed Drilled Shaft Inspection Report for each drilled shaft, detailing the actual location, alignment, elevations, dimensions, and quantities of the shafts. Submit the report within 21 calendar days after the completion and acceptance of each shaft. A "Drilled Shaft Inspection Report" form is available from the Engineer.

(d) Concrete Placement Logs and Volume Curves - Measure and record all concrete placed into drilled shafts using standard ODOT forms designated for this purpose or other forms approved by the Engineer. Provide the Engineer with a completed Drilled Shaft Concrete Placement Log and Concrete Volume Curve Form for each drilled shaft within 24 hours after completion of shaft concrete placement.

Concrete Placement

Arrival of Concrete

Documentation

Truck No.	Concrete Volume	Slump	Arrival Time	Start Time	Finish Time	Tremie Depth	Depth To Concrete	NOTES (delays, additives, breaching, casing removal)
1	8 cy		7:25	7:40	7:55	48	42	Initial QCT concrete test
2	8 cy		7:40	8:00	8:10	48	39	
3	8 cy		8:00	8:20	8:50	48	35	
4	8 cy		8:45	9:05	9:25	38	32	Second QCT concrete test
5	8 cy		9:10	9:30	9:50	38	28	
6	8 cy		9:50	10:00	10:10	28	20	Truck was delayed by traffic
7	8 cy		10:00	10:15	10:45	28	19	talked to contractor about the depth of concrete
8	8 cy		10:40	10:50	11:20	28	19	talked to contractor about the depth of concrete
9	8 cy		11:00	11:30	11:40	28	15	Truck was delayed at plant
10	8 cy		11:20	11:50	12:20	18	10	Third QCT concrete test
11	8 cy		12:25	12:30	12:50	18	5	
12	8 cy		12:40	1:00	1:45	18	0	Finished pour

96 cy Total Concrete Volume Delivered (TVD)



OSSC2015

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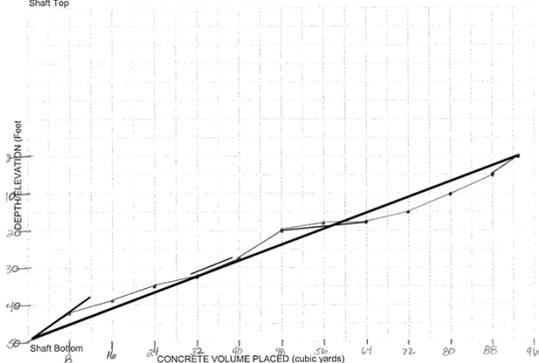
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Concrete Placement

Documentation

CONCRETING CURVE

Prior to pouring concrete, a plot should be made showing the theoretical concrete surface (by depth or elev.) vs. concrete volume placed. During concrete placement the actual concrete surface vs. the actual concrete volume placed is then plotted.



Concrete Placement

For example

- If you have a 5-gallon gas can but during filling up it takes only 3 gallons there is a problem. Something is taking up space in the can has partially collapsed/dented. This is like a hole wall cave-in a drilled shaft.
- If filling the gas can and it takes more than 5 gallons your feet are wet, you notice leaking. This is like a void in a drilled shaft.



Concrete Placement Equations, Last One!

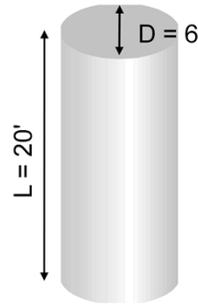
Volume of a shaft = Area x Length

- $V = \frac{\pi d^2}{4} \times L$

- To determine the volume of a shaft with a diameter of 6' that is 20 feet long:

$$(A = \frac{3.14 \times 6^2}{4}) \times 20' = 565.20 \text{ cubic feet}$$

- $565.20 \text{ cf} \times 1 \text{ cy}/27 \text{ cf} = 20.93 \text{ cy}$



Calculate Theoretical Volume of Concrete

- Diameter = 8'
- Shaft Length = 50'
- Volume = $\frac{\pi d^2}{4} \times \underline{L}$
- Volume = $\frac{3.14 \times 8^2}{4} \times 50$
- Volume = 2,512 cubic feet
- **Volume = 2,512 cf/27 = 93 cy**



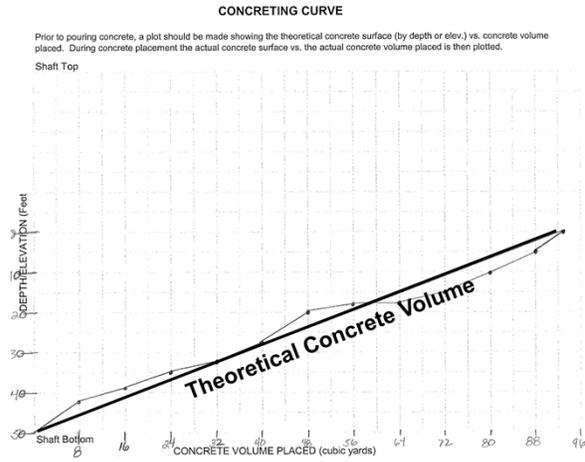
*If the shaft is perfectly shaped, it will take 93 cy to fill it completely.
Are drilled shafts ever perfect?*

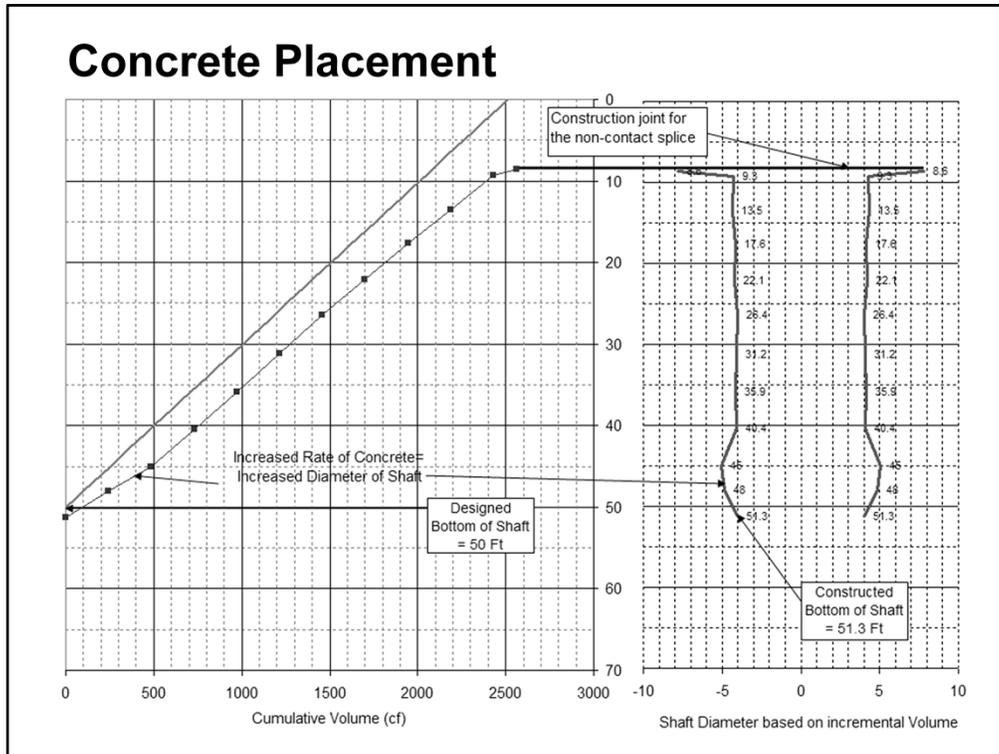


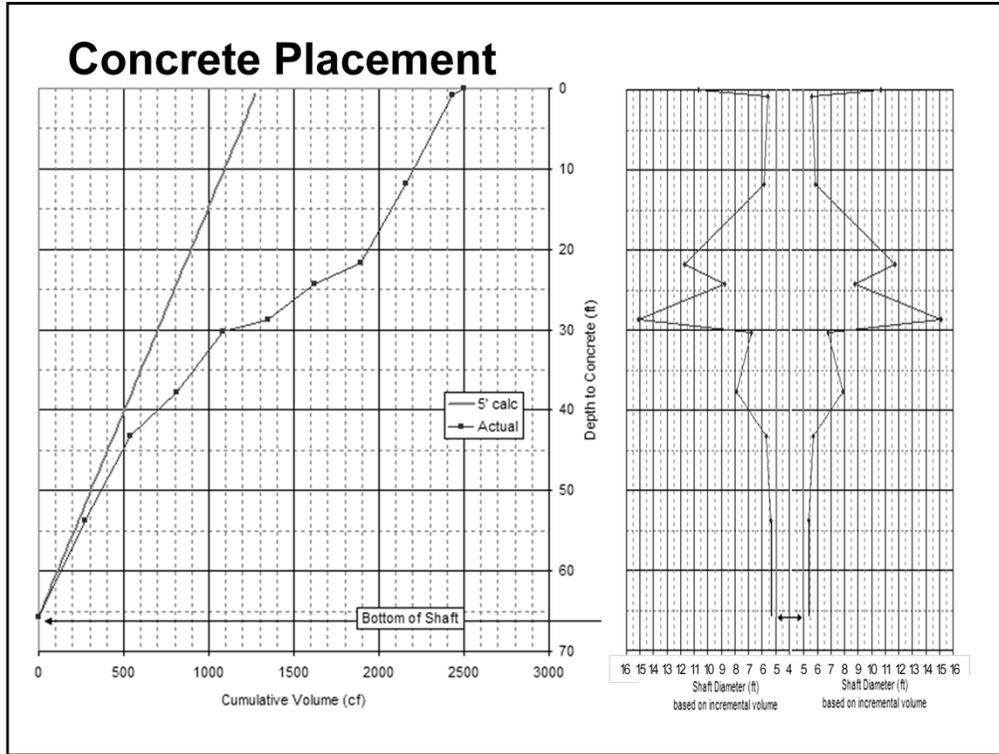
Concrete Placement

So what does it mean?

- Theoretical line represents Volume (cy)/LF
- At Calapooya 1.86 cy/LF







Concrete Placement Conversions

1 foot = 12"

- To convert 3" to feet:
- $3" \times 1 \text{ foot}/12" = 0.25 \text{ feet}$

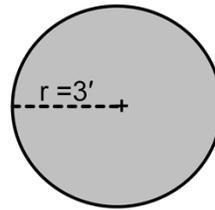
1 yard = 3'

- To convert 4' to yards:
- $4' \times 1 \text{ yard}/3' = 1.33 \text{ yards}$



Concrete Placement Equations (continued)

- Area of a circle = $\pi \times r^2$
- To determine the area of a circle with a diameter of 6':
 $3.14 \times 3^2 = 28.26 \text{ sf}$



Concrete Placement Conversions

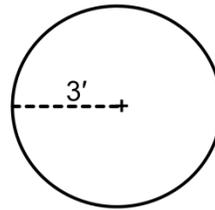
1 cubic yard = 27 cubic feet

- To convert 300 cubic feet to cubic yards:
- $300 \text{ cubic feet} \times 1 \text{ cubic yard} / 27 \text{ cubic feet}$
= 11.11 cubic yards



Concrete Placement Equations

- Circumference of a circle = $\pi \times \text{Diameter}$
- Diameter = Radius $\times 2$
- To determine the circumference of a circle with a diameter of 6': $6 \times 3.14 = 18.84'$



Concrete Placement

EXAMPLE Class Drilled Shaft				12345		CON10000		
BENT	STATION			SHAFT NO.	SHAFT DIAMETER			
2	100+00			#1	6 feet			
DRILLED SHAFT CONTRACTOR				INSPECTED BY	CERT. NO.	DATE		
Diggin Deep Drilled Shaft Construction				Abby Normal	41071	2/1/2013		
REFERENCE ELEVATION	SHAFT TOP ELEVATION			REBAR CAGE TOP ELEVATION:		AT START	AT FINISH	
315	310					314.9	315.8	
DEPTH TO WATER OR SLURRY	SHAFT BOTTOM ELEVATION			REBAR DESIGN ELEV.	WITHIN SPEC? <input checked="" type="checkbox"/> YES <input type="checkbox"/> NO			
N/A	245			315.5				
TOP OF ROCK ELEVATION	SHAFT LENGTH			REBAR CAGE CENTERED WITHIN SPEC? <input checked="" type="checkbox"/> YES <input type="checkbox"/> NO				
266.9	65							
SHAFT CONCRETE INFORMATION								
Placement Method		Volume in Lines			Begin Pour: Date: 7/18/2013 Time: 8:00 AM			
YES Free Fall	#	ID	Length	Volume	End Pour: Date: 7/18/2013 Time: 11:15 AM			
NO Tremie	0	0		0.0 cy	Shaft Completion Time: 11:45 AM (including casing removal)			
De-Airing Method					cy			
NO Tremie Plug				cy	75 cyd	Total Concrete Volume Delivered (TVD)		
NO Tremie Cap	Total Volume in Lines (VL)			cy	74 cyd	Total Concrete Volume in Shaft; cy (=TVD-VL-VW)		
NO Relief Valve	Estimated Waste Concrete (VW)			1	cy			
Truck No.	Concrete Volume	Slump	Arrival Time	Start Time	Finish Time	Tremie Depth	Depth To Concrete	NOTES (delays, additives, breaching, casing removal)
1	10 cy	9.8	7:50 AM	8:00 AM	8:15 AM	55.5	58.75	Initial QCT concrete test - passes
2	10 cy	9.5	8:00 AM	8:20 AM	8:28 AM	51	53.25	Talked to contractor about depth of concrete
3	10 cy	10	8:15 AM	8:35 AM	8:50 AM	42	42.5	Second QCT concrete test - passes
4	10 cy	9.1	8:45 AM	8:55 AM	9:05 AM	35	35	
5	10 cy	9.6	8:50 AM	9:10 AM	9:30 AM	25	25	Talked to contractor about depth of concrete
6	10 cy	10.2	9:20 AM	9:40 AM	9:55 AM	10	10	Third QCT concrete test - See notes below
7	10 cy	9.9	9:40 AM	10:00 AM	10:35 AM	3	3	Finished pour
8	5 cy	9.5	10:30 AM	10:50 AM	11:15 AM	0	0	
								Truck #6 notes: slump 0.2 out of specifications. Contractor opted to pour. Confirmed with assistant PM to continue

Concrete Placement



Truck No.	Concrete Volume	Slump	Arrival Time	Start Time	Finish Time	Tremie Depth	Depth To Concrete	NOTES (delays, additives, breaching, casing removal)
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7	10 cy	9.9	9:40 AM	10:00 AM	10:35 AM	3	3	Finished pour
8	5 cy	9.5	10:30 AM	10:50 AM	11:15 AM	0	0	
								Truck #6 notes: slump 0.2 out of specifications. Contractor opted to pour. Confirmed with assistant PM to continue



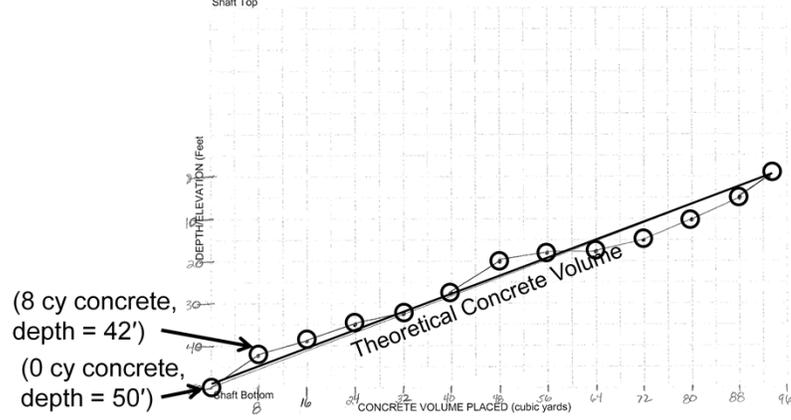
Concrete Placement

Documentation

CONCRETING CURVE

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Shaft Top



Inspector's Roles and Responsibilities

What are the Inspector's Roles and Responsibilities?

- Communicate
- Document



Inspector's Roles and Responsibilities

Documentation

Drilled Shaft Placement Log 00512.40

Concrete Placement Logs and Volume Curves –

Measure and record all concrete placed into drilled shafts using standard ODOT forms designated for this purpose or other forms approved by the Engineer. Provide the Engineer with a completed Drilled Shaft Concrete Placement Log and Concrete Volume Curve Form for each drilled shaft within 24 hours after completion of shaft concrete placement.



- It is the contractors' responsibility to complete the Concrete Placement Logs and Volume Curves.
- The drilled shaft inspector will keep ODOT's own set of records.
- It is the Inspector's responsibility to document on ODOT's behalf.

Inspector's Roles and Responsibilities

Concrete Operations

- Yes No NA 31. Prior to concrete placement, has the slurry (both manufactured and natural) been tested in accordance with Section 00512.43(g)??
- Yes No NA 32. If required, was the casing removed in accordance with Section 00512.47(e)?
- Yes No NA 33. Does the Contractor's tremie meet the requirements of Section 00512.47(a)?
- Yes No NA 34. Was the discharge end of the tremie maintained in the concrete mass with proper concrete head above it at all times (00512.47(c))?
- Yes No NA 35. For shafts with non-contact splices, have the cold joints been properly cleaned and roughened in accordance with Section 00512.47(a)?
- Yes No NA 36. For shafts without non-contact splices, did the Contractor overflow the shaft until good concrete flowed out of the top of the excavation (00512.47(a))?
- Yes No NA 37. Have the Concrete Placement and Concrete Volume logs been completed?
- Yes No NA 38. Were the concrete acceptance tests performed as required?
- Yes No NA 39. Were the Crosshole Sonic Log (CSL) tubes filled with water and capped in accordance to Section 00512.46?



Concrete Operations Learning Objectives

- Concrete basics
 - mix design
 - concrete placement
 - Concrete sampling and testing
- Concrete Placement Log
- Concrete Volume Log
- Inspector's roles, and responsibilities

